# PROPOSED ATTACHED DUAL OCCUPANCY AT 166 FLUSHCOMBE ROAD, BLACKTOWN

#### **GENERAL NOTES**

- G1 These drawings shall be read in conjunction with other consultants' drawings and specifications and with other such written instructions as may be issued during the course of the Contract. Any discrepancy shall be referred to the Engineer before proceeding with the work.
- G2 All dimensions are in millimetres, U.N.O. (unless noted otherwise)
- G3 No dimension shall be obtained by scaling the drawings.
- G4 All levels and setting out dimensions shown on the drawings shall be checked on site prior to the commencement of the work.
- G5 During construction the structure shall be maintained in a stable condition and no part shall be overstressed.
- G6 Damp-proofing & sealing details shall be in accordance with Architect's details. All joints in concrete elements shall be suitably sealed or damp-proofed

#### **FOUNDATIONS**

Refer Slab Design Criteria for classification of site.

- F2 Footings have been designed for an allowable bearing pressure of 150 kPa U.N.O. All foundations must be stable and uniform throughout.
- F3 Foundation material shall be inspected and approved for the above site classification and allowable bearing pressure by a Geotechnical Engineer before placing footing reinforcement.
- F4 Footings shall be placed centrally under walls and columns, U.N.O.

#### LOADING

Superimposed floor loads are generally in accordance with AS 1170.1 or as noted

Wind loads are in accordance with AS/NZS 1170 2 as follows:

Region: A 2 Regional Wind Velocity, V500: 45 m/s Category: 3, U.N.O. Earthquake loads are in accordance with AS 1170.4 as follows:

a = 0.08 S = 1.0 I = 1.0. U.N.O

Live loads & additional dead loads: (to AS/NZS 1170.1)

Area subject to	Live	Load	Add. Dead	
loading	Uniform	Point	Load	
Floors - Internal	1.50 kPa	1.80 kN	0.50 kPa	
Floors - External & Garage	3.00 kPa	1.80 kN	1.00 kPa	
Roof Areas	0.25 kPa	1.40 kN	0.15 kPa	

#### **MASONRY**

- M1 All workmanship and materials shall be in accordance with AS 3700.
- M2 Characteristic compressive strength of masonry (fuc) = 24 MPa

Durability	Durability Requirements						
Mortar	Salt Attack Resistance Grade	Built In Component	Min. Cover to Reinforcement & Tendons in Grouted Cavities				
M2	Protected	R1 (Galv'd 300 g/m² each side)	5				
M3	General Purpose	R3 (Galv'd 470 g/m² each side)	15				
M4	Exposure	R4 (Stainless)	30				

- M3 All masonry walls supporting slabs and beams shall have a pre-greased two lavel galvanised steel slip joint between concrete and masonry.
- M4 All masonry walls supporting or supported by concrete floors shall be provided with vertical joints to match any control joints in the concrete.
- M5 Non load bearing walls shall be separated from concrete above by 12 mm thick closed cell polyethylene strip. M6 Provide vertical control joints at 8 metres maximum centres, and 4 metres
- maximum from corners in masonry walls, and between new & existing brickwork.
- M7 Masonry retaining walls are to be backfilled with either of the following material:
  - Coarse grained soil with low silt content
    - Residual soil containing stones
    - Fine silty sand
    - Granular materials with low clay content

#### REINFORCED CONCRETE

- C1 All workmanship and materials shall be in accordance with AS 3600 current edition, except where varied by the contract documents
- C2 Concrete quality shall be as follows (subject to note C4 being satisfied):

Element	Slump mm	Max. Agg. Size mm	Cement Type	fc at 28 Days MPa
Footings	80	20	Normal	20
Slabs on Ground	80	20	Portland	25
Suspended Floors	80	20	Type A	32

- C3 Engineer to approve any admixtures used in concrete mix.
- C4 Cover to reinforcement shall be obtained by the use of approved bar chairs. All chairs to be placed at 750 maximum centres

Minimum clear concrete cover to reinforcement including ties and stirrups (other than residential slabs on ground or footings) shall be as follows uno.

Exposure Classification	Minimum Cover (mm)					
	Concrete Strength (fc)					
	20 MPa	25 MPa	32 MPa	40 MPa	>50 MPa	
A1	20	20	20	20	20	
A2	-	30	25	20	20	
B1	-	-	40	30	25	
B2	-	-	-	45	35	
С	-	-	-	-	50	

- Residential slab on ground and footings cover requirements:
  - (Minimum concrete grade N20)
    - Unprotected ground: 40 mm
    - External exposure: 40 mm - Membrane in contact with ground: 30 mm
    - Internal surface: 20 mm
    - Strip & pad footing: 40 mm
- All concrete shall be mechanically vibrated. Vibrators shall not be used to spread
- Sizes of concrete elements do not include thickness of applied finishes.
- No holes or chases other than those shown on the structural drawings shall be made in concrete members without the prior approval of the Engineer
- C10 Construction joints where not shown shall be located to the approval of the
- C11 Curing of all concrete is to be achieved by keeping surfaces continuously wet for a period of 3 days, and prevention of loss of moisture for a total of 7 days followed by gradual drying out. Approved sprayed on compounds may be used where no floor finishes are proposed. Polythene sheeting or wet hessian may be used if protected from wind and traffic.
- C12 Construction support propping is to be left in place where needed to avoid over stressing the structure due to construction loading. No masonry or partition walls are to be constructed on suspended levels until all propping is removed and the slab has absorbed its dead load deflection
- C13 Conduits, pipes, etc. shall only be placed in the middle one third of slab depth and spread at not less than 3 diameters
- C14 Reinforcement symbols :
  - Denotes deformed grade 500 normal ductility reinforcing bars to AS/NZS 4671.
    - Denotes plain round grade 250 normal ductility reinforcing bars to AS/NZS 4671.
  - Denotes deformed grade 500 low ductility reinforcing mesh to AS/NZS 4671.
  - Denotes deformed grade 500 low ductility reinforcing mesh to AS/NZS 4671.
  - L--TM Denotes deformed grade 500 low ductility trench mesh to AS/NZS 4671.
- C15 Reinforcement is represented diagrammatically; it is not necessarily shown in true
- C16 Splices in reinforcement shall be made only in positions shown or otherwise approved by the Engineer.
- C17 Fabric reinforcement shall have splices made so that the overlap, measured between the outermost transverse wires of each sheet of fabric, is not less than the spacing of those wires plus 25 mm.
- C18 Welding of reinforcement shall not be permitted unless shown on the structural drawings or approved by the Engineer

#### STRUCTURAL STEEL

- All workmanship and materials shall be in accordance with AS 4100, AS 1163. AS 1554.1 and AS/NZS 4600.
- The structural design has been based on the following steel grades, U.N.O: Hot rolled universal beams, columns, channels & angles:300PLUS Circular, square & rectangular hollow sections: Cold formed open DuraGal profiles: C400/C450LO Cold formed lipped Cee & Zed purlins: G550/G500/G450
- The structural design has been based on MBPMA nominal size Cee & Zed lipped purlins. All purlin profiles shall be in accordance with the MBPMA specifications.
- Qualifications of welding procedures and personnel shall conform to Section 4 of AS 1554.1. Non destructive testing of welds shall include 100% visual inspection and additional testing as shown on the drawings.

  All welds shall be 6 mm continuous fillet type SP, U.N.O. All butt welds shall be complete penetration in accordance with AS 1554.1, U.N.O.

Commercial bolts to AS 1111, snug tightened High strength structural bolts to AS 1562, snug tightened High strength structural bolts to AS 1562, fully tensioned bearing joint to AS 1511 High strength structural bolts to AS 1562, fully tensioned friction joint

All bolts shall be M16 8.8/S, with a minimum of 2 bolts per connection, U.N.O.

- S7 High strength TF & TB bolts shall be installed using approved load indicator washers, or in accordance with the part turn method nominated in AS 4100. S8 Gusset plates shall be 10 mm thick, grade 300PLUS steel, U.N.O.
- S9 Concrete encased steelwork shall be wrapped with SL41 fabric and shall have a minimum of 50 mm cover UNO
- S10 Steelwork not encased shall have the following surface treatment

,					
	Exposure Classification	Steelwork Protection Required			
	A1 / A2	Power tool clean to AS1627 Class 1 1 Coat Alkyd Primer (Zinc Phosphate)			
	B1	Abrasive blast to AS1627 Class 2.5 1 Coat Inorganic Zinc Silicate			
	B2	Hot Dipped Galvanised to AS4680			

- S11 Where sealed tube members are hot dip galvanised, the fabricator shall provide drill holes as necessary.
  S12 All transport and erection damage, site welds etc., shall be reinstated to
- an equivalent finish to adjacent steelwork

#### SITE PREPARATION FOR SLABS ON GROUND

- P1 Strip topsoil containing organic matter. Proof roll fill sub grade and remove any
- P2 Where additional fill is required to the underside of slabs on ground, non cohesive materials such as sand and gravel dust shall be placed by "controlled" compaction in horizontal layers of 200 mm (loose) maximum depth. This fill shall be compacted to at least 95% of Standard Maximum Dry Density (SMDD).
- P3 For slabs on ground, sand 50 mm approximate thickness is to be spread as a levelling layer and well watered down.
- P4 Damp-proofing membrane unpunctured and taped at laps, is to be placed over the sand, sufficient membrane being provided at edges to return under brickwork. Where no brickwork, tape membrane to side of footing below ground.

#### FOUNDATION MAINTENANCE

FOUNDATION SOILS: All soils are affected by water. Silts are weakened by water and some sands can settle if heavily watered, but most problems arise on clay foundations. Clays swell and shrink due to changes in moisture content and the potential amount of the movement is implied in the site classification in Australian Standard AS2870, which is specified as follows:

- A Stable (Non-reactive).
- S Slightly Reactive.
- M Moderately Reactive
- H Highly Reactive. E Extremely Reactive

CLASS A & S SITES: Sands, silts and clays shall be protected from becoming extremely wet by adequate attention to site drainage and prompt repair of plumbing

CLASS M, H & E SITES: Sites classified as M, H, or E shall be maintained at essentially stable moisture conditions and extremes of wetting and drying prevented. This will

Drainage of the site: The site shall be graded or drained so that water cannot pond against or near the house. The ground immediately adjacent to the house shall be graded to a uniform fall of 50 mm minimum away from the house over the first metre. The sub floor space for houses with suspended floors shall be graded or drained to prevent ponding where this may affect the performance of the footing system The site drainage requirements shall be maintained for the economic life of the building

Limitations on gardens: The development of the gardens shall not interfere with the drainage requirements or the sub floor ventilation and weep hole drainage systems Garden beds adjacent to the house should be avoided. Care should be taken to avoid over watering of gardens close to the house footings

Restrictions on trees and shrubs: Planting of trees should be avoided near the foundation of a house or neighbouring house on reactive sites as they can cause damage due to drying of the clay at substantial distances. To reduce, but not eliminate, the possibility of damage, tree planting should be restricted to a distance

- 1.50 x mature height for Class E sites
- 1.00 x mature height for Class H sites 0.75 x mature height for Class M sites

Where rows or groups of trees are involved, the distance from the building should be increased. Removal of trees from the site can also cause similar problems.

Repair of leaks: Leaks in plumbing, including storm water and sewerage drainage

The level to which these measures are implemented depends on the reactivity of the site. The measures apply mainly to masonry houses and masonry veneer houses. For frame houses clad with timber or sheeting, lesser precautions may be appropriate

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A: SUITE 4, LEVEL 1, 402-410 CHAPEL RD, BANKSTOWN, NSW 2200 P: 9709 5556 M: 0422 606 228

166 FLUSHCOMBE RD, BLACKTOWN **GENERAL NOTES** 

PROPOSED ATTACHED DUAL OCCUPANCY

BONDEK/CONDECK FORMWORK

at all masonry u.n.o.

B1 U.N.O. Bondek/condeck panels shall be 1.00mm bmt

methods. slip joints shall be located as shown

construction loading or wind uplift prior to concreting

or exposed soffits additional fixing may be required

Panels are to be securely fixed or held down to prevent displacement due to

Fxing to masonry is not necessary provided concrete is placed immediately

after panels are laid. Top course of brickwork is to be straight and level. If

required, provide layer of smooth, hard mortar slip joints shall be provided

other substance will need to be removed to ensure clean bonding surface.

Fastening of side lap joints shall be in accordance with lysaght publications, and generally one no. 10-24x16mm self-drilling tapping screw is required

mid-span for support spacing of 2750mm or greater. For point loads ratings

Props shall not be removed until concrete has reached sufficient strength

Fix panels to steelwork by puddle welding drive pins or other suitable

Before concrete is placed, any accumulated debris, grease or any

U.N.O Propping shall be in accordance with lysaght publications

Any ponded rainwater should be removed by blowing or sweeping

JOB NUMBER: 24244	DWG NUMBER: S01	
DESIGNED BY: A.N.	DATE: 03.09.2024	
DRAWN BY: A.N.	SCALE: AS SHOWN	

NOTE: DO NOT SCALE OFF DRAWINGS, REFER TO

STRUCTURAL ENGINEERING

#### **LEGEND**

\_\_\_\_\_ DENOTES CORE FILLED BLOCK WALL OVER

DENOTES NON LOAD BEARING WALL OVER

= = = DENOTES NON LOAD BEARING WALL UNDER

DENOTES LOAD BEARING MASONRY WALL UNDER

DENOTES LOAD BEARING 190 DINCEL WALL 

DENOTES SLAB PENETRATION

XX DENOTES SLAB STEP DEPTH

??? DENOTES MINIMUM SLAB DEPTH

\_ sc^ DENOTES STEEL COLUMN OVER DENOTES STEEL COLUMN UNDER

DENOTES STEEL COLUMN UNDER + OVER

DENOTES SLAB DATUM

SB1(-150) DENOTES STEEL BEAM 150mm BELOW SLAB DATUM

DENOTES SAWN JOINT. REFER TO DETAILS.

DENOTES KEY JOINT, REFER TO DETAILS.

- 0<sup>5</sup> DENOTES DOUBLE STUD

DENOTES TRIPLE STUD <u> مرح</u>

PS DENOTES F11x4.5 THICK PLYWOOD SHEET STRUCTURAL BRACING. REFER TO TIMBER FRAMING CODE FOR FIXING.

MSX DENOTES 30x0.8 METAL STRAP CROSS BRACING. REFER TO TIMBER FRAMING CODE AS1684

#### TIMBER FRAMING NOTES

- ALL DESIGN. WORKMANSHIP & MATERIALS SHALL BE IN ACCORDANCE WITH NATIONAL TIMBER FRAMING CODE AS1684 CURRENT EDITION WITH AMENDMENTS, EXCEPT WHERE VARIED BY THE CONTRACT DOCUMENTS.
- TIMBER SIZES, CONNECTIONS AND BRACING WALL SHALL BE TO FRAME MANUFACTURER'S DETAILS & SPECIFICATIONS & SHALL BE IN ACCORDANCE WITH AS1684. TIMBER FRAMING OUTSIDE THE SCOPE OF AS1684 SHALL BE REFERED TO THE SUPERINTENDENT FOR A DECISION BEFORE PROCEEDING.
- DRAWINGS SHALL BE IN ACCORDANCE WITH AS1684.
- TRUSS LAYOUT TO BE FORWARDED TO ANH CONSULTING TF5. ENGINEERS BEFORE COMMENCEMENT OF WORK ON SITE
- MINIMUM 3 COURSES OF BRICK TO BE LAID ABOVE LINTELS
- TF7 FOR LINTEL NOT SHOWN ON PLAN, REFER TO STANDARD LINTEL SCHEDULE

- TF3. ROOF BRACING & ANCHOR DETAILS WHERE NOT SHOWN ON
- ROOF TRUSSES TO MANUFACTURERS SPECIFICATION
- TF8. FIX ALL STUD WALLS TO STEEL COLUMNS

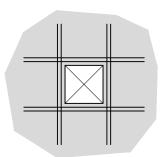
## 110

DENOTES EXTENT OF 110 THICK SLAB ON GROUND REINFORCED WITH SL82 MESH TOP CONTINUOUS THROUGHOUT PLUS ANY EXTRAS AS NOTED ON PLAN AND IN DETAILS.



DENOTES EXTENT OF 150 THICK 400mm MAXIMUM COMPACTED FILL REINFORCED WITH SL82 MESH TOP & BTM CONTINUOUS THROUGHOUT PLUS ANY EXTRAS AS NOTED ON PLAN AND IN DETAILS.

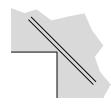
#### TYPICAL SLAB TRIMMER DETAILS



SLAB ON GROUND NOTES

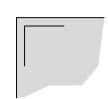
PROVIDE 2N12 BARS TOP EXTEND 600 MIN. PAST EACH EDGE OF THE PENETRATION.

#### SLAB PENETRATION TRIMMER



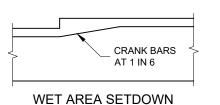
2N12-1200 LONG (75 SPACING) TRIMMER BARS AT ALL RE-ENTRANT CORNERS, TIED TO UNDERSIDE OF TOP REINFORCEMENT.

#### SLAB RE-ENTRANT CORNER TRIMMER

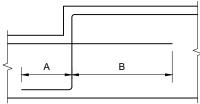


1N12 L-BARS TOP IN CORNER OF SLAB, 1000 LEGS.

#### SLAB CORNER TRIMMER

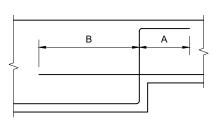


STANDARD CO SCHEDULE	G 900
BAR	COG (mm)
UP TO N12	200
N16	200
N20	245
N24	295
N28	340
N32	390
N36	440



TOP OF SLAB STEP

TYPICAL REINFORCEMENT BAR LAP AT STEPS						
BAR DIA.	Α	В				
N12	200	600				
N16	300	800				
N20	400	1000				
N24	500	1200				
N28	600	1400				
N32	700	1600				
N36	N36 800 1800					
	•					



#### **BOTTOM OF SLAB STEP**

SLAB REINFORCEMENT MIN. SPLICE SCHEDULE			
BAR	LAP (mm)		
UP TO N12	480		
N16	640		
N20	800		
N24	960		
N28	1120		
N32	1280		
N36	1440		



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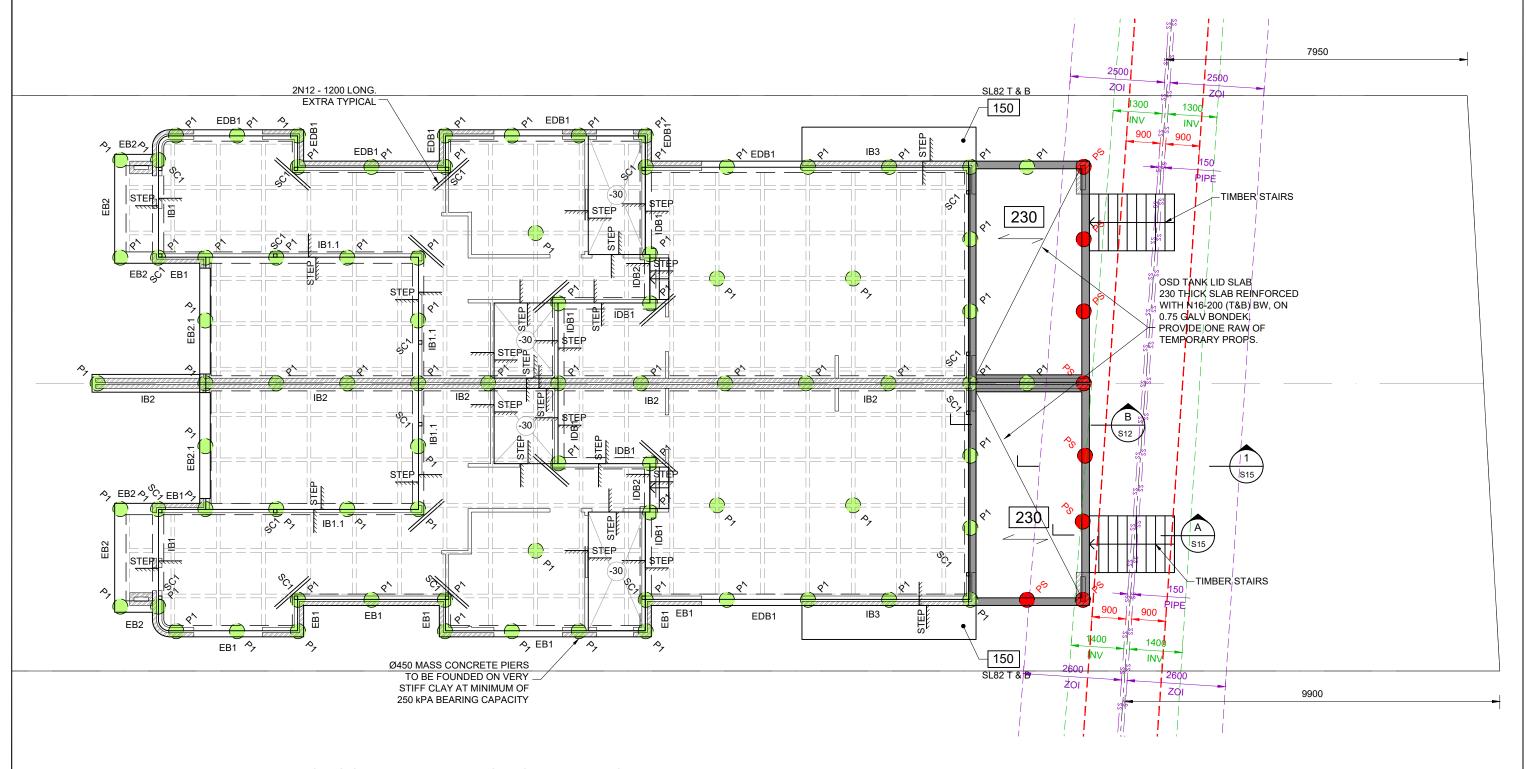
STRUCTURAL ENGINEERING & ARCHITECTURAL DESIGN

PROPOSED ATTACHED DUAL OCCUPANCY 166 FLUSHCOMBE RD, BLACKTOWN

LEGEND AND PROJECT SPECIFICATION

JOB NUMBER: 24244	DWG NUMBER: \$02	ORIGINAL SIZE:
DESIGNED BY: A.N.	DATE: 03.09.2024	
DRAWN BY: A.N.	SCALE: AS SHOWN	

NOTE: DO NOT SCALE OFF DRAWINGS, REFER TO ARCHITECTURAL PLANS, VERIFY DIMENSIONS ON SITE



## PROPOSED WAFFLE POD SLAB LAYOUT PLAN

#### NOTES:

- ALL FOOTINGS TO BEAR ON SAME STRATA & ON NATURAL GROUND
- ARTICULATE ALL BRICKWORK & DRAINAGE TO BCA
- BUILDER TO REPORT SITE CONDITION TO STRUCTURAL ENGINEER AFTER THE SITE CUT & FILL IS COMPLETE
- 4. DESIGN LOADING AT FOOTING LEVEL NOT EXCEEDING 50kPa

#### **GEOTECHNICAL NOTES:**

PIERS AND SLAB HAVE BEEN DESIGNED IN ACCORDANCE WITH AN ASSUMED SOIL CLASS INDICATED IN THE DESIGN CRITERIA. TO OPTIMISE SLAB& PIERING CONFIGURATION, NEMCO DESIGN RECOMMEND GEOTECHNICAL ASSESSMENT OF THE SITE.

WHERE UNCONTROLLED FILL IS ENCOUNTERED, GEOTECHNICAL INVESTIGATION REQUIRED.



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PROPOSED ATTACHED DUAL OCCUPANCY	JOB NUMBER: 24244	S10.2	A3	
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	DESIGNED BY: A.N.	DATE: 03.09.2024		
PROPOSED WAFFLE POD SLAB LAYOUT PLAN	DRAWN BY: A.N.	SCALE: AS SHOWN		

NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE

1	MENT NCRETE QUALITY	STRENGTH f'c	MAX SIZE AGG. mm	SLUMP mm	CEMENT TYPE	ADMIXTURE
WAF	FLE POD SLAB	25	20	80	GP	-
PIER	S	25	20	80	GP	-

REFER TO GENERAL NOTES FOR REINFORCEMENT COVER

#### NOTE:

REFER TO DRAWING S02 FOR LEGEND & PROJECT SPECIFICATION

#### LEGEND



DENOTES LOCATION OF 300 DEEP PODS. POD SIZE: 1090 x 1090 (CUT PODS AS REQUIRED)



DENOTES LOCATION OF 225 DEEP PODS. POD SIZE: 1090 x 1090 (CUT PODS AS REQUIRED)



POD LAYOUT STARTING POINT



P1 DENOTES Ø450 CONC UNO. PIERS. REFER TO BORED PIER NOTES AND DETAIL ON DWG S14



PS DENOTES Ø450 CONC. PIERS MUST BE INSPECTED BY SYDNEY WATER PRIOR TO PLACEMENT OF CONCRETE.

#### WAFFLE SLAB NOTES

85 THICK SLAB REINFORCED WITH SL82 MESH TOP CONTINUOUS THROUGHOUT INCLUDING ANY EXTRAS AS NOTED ON PLAN OR IN DETAILS.

SITE PREPARATION SHALL BE CARRIED OUT IN ACCORDANCE WITH THE GENERAL NOTES & CURRENT EDITION OF AS2870 - RESIDENTIAL SLABS & FOOTING CODE.

REFER TO GENERAL NOTES FOR CONCRETE COVERS

TYPICAL WAFFLE POD LAYOUT AT RE-ENTRANT **CORNER REFER TO DRAWING S13** 

#### **DESIGN CRITERIA**

CONSTRUCTION.

SITE SOIL CLASSIFICATION : ASSUMED CLASS M (REFER TO NOTE BELOW) SITE WIND CLASSIFICATION: N2 CONSTRUCTION TYPE: BRICK VENEER ROOF FRAMING: SKILLION ROOF

NOTE:
THE SUPERINTENDENT SHALL HAVE THE SITE SOIL CLASSIFICATION CONFIRMED (BY INSPECTION OF TEST PIER HOLE 1500 MIN DEEP OR TO AUGER REFUSAL, WHICHEVER IS LESS) BY THE ENGINEER PRIOR TO COMMENCING CONSTRUCTION. WHERE THE CLAY EXTENDS FOR 1500 OR MORE THE SUPERINTENDENT SHALL HAVE THE SITE CLASSIFICATION CONFIRMED BY A SUITABLY QUALIFIED GEOTECHNICAL ENGINEER PRIOR TO COMMENCING

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STRUCTURAL ENGINEERING & ARCHITECTURAL DESIGN

PROPOSED ATTACHED DUAL OCCUPANCY 166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939

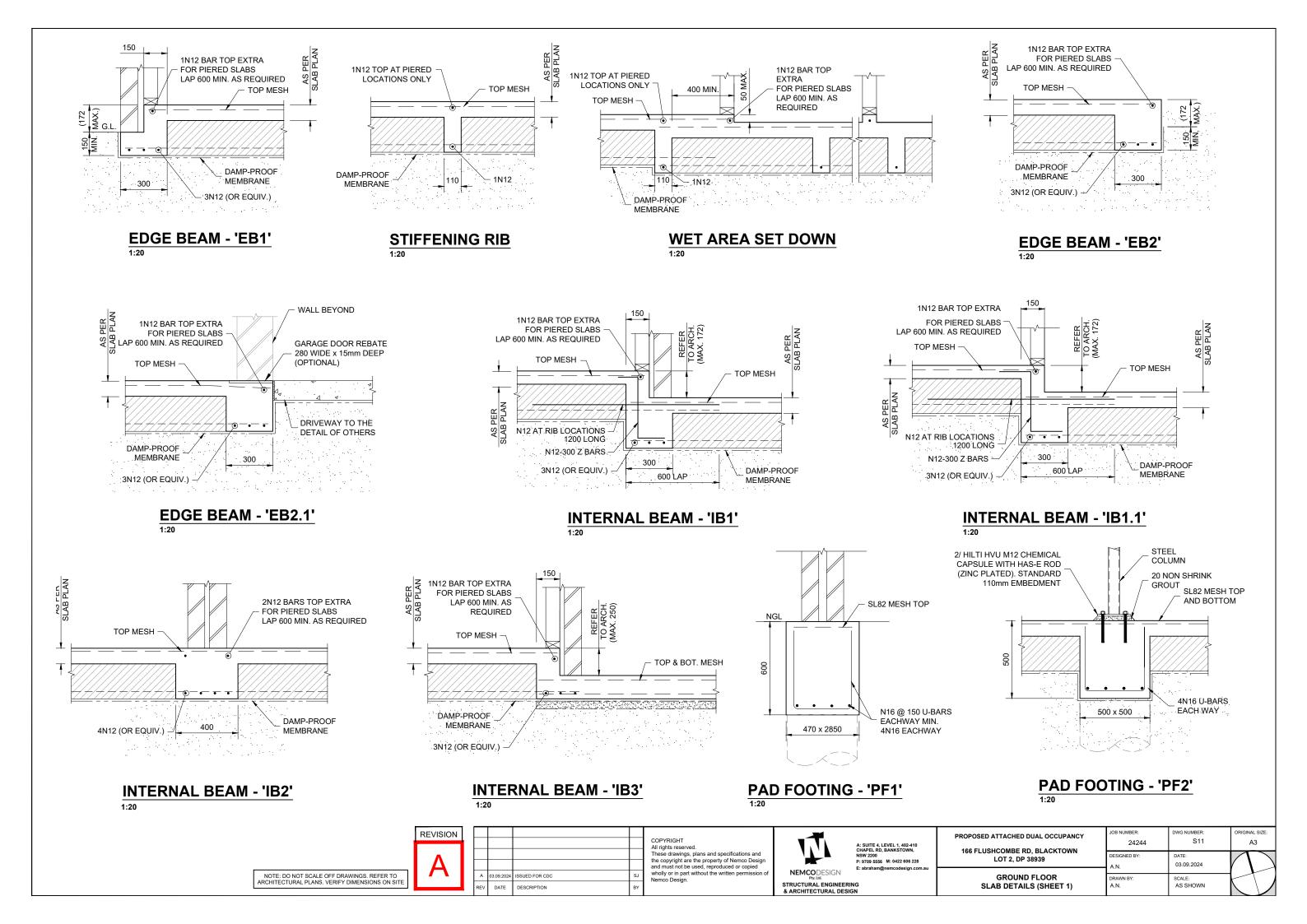
LAYOUT PLAN

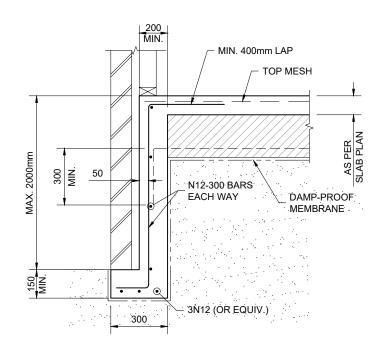
PROPOSED WAFFLE POD SLAB

OB NUMBER: S10.3 24244 DESIGNED BY: 03 09 2024 RAWN BY: SCALE: AS SHOWN

A3

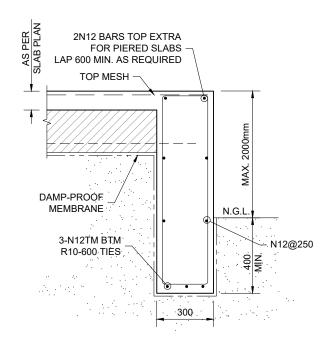
NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE



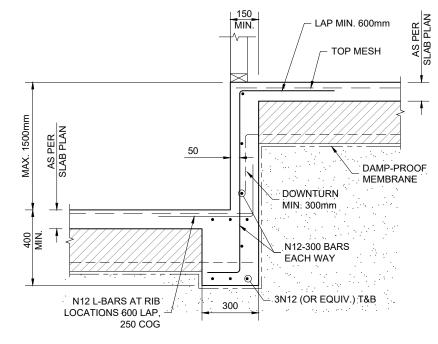


# EDGE DROP BEAM - 'EDB1'

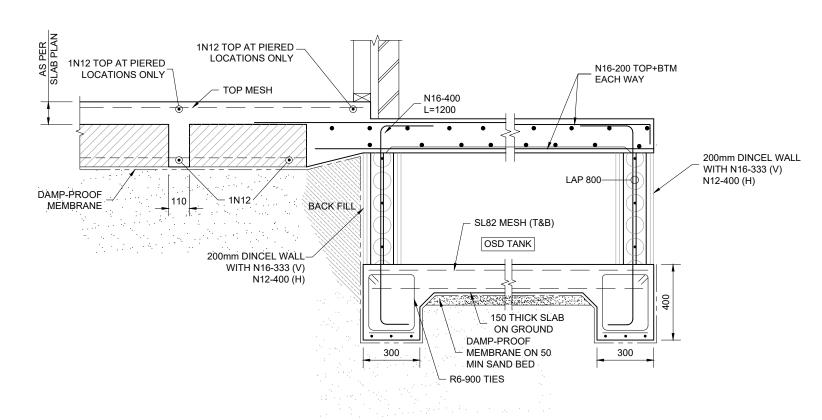
# INTERNAL DROP BEAM - 'IDB2'



# EDGE DROP BEAM - 'EDB2'



# INTERNAL DROP BEAM - 'IDB1'







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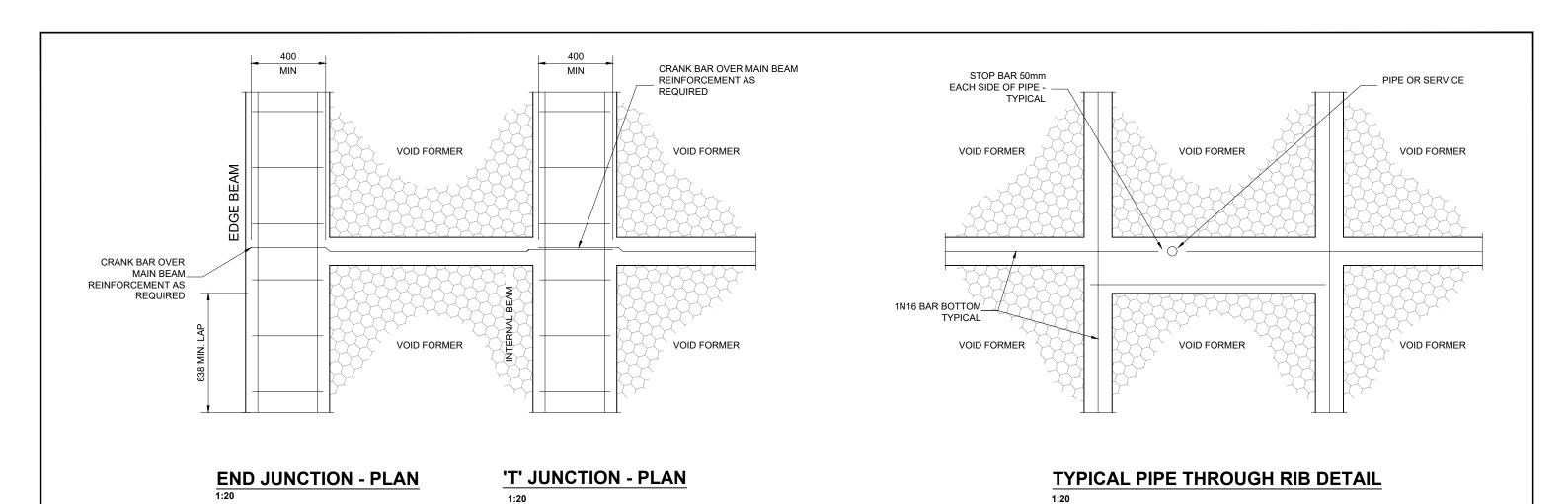
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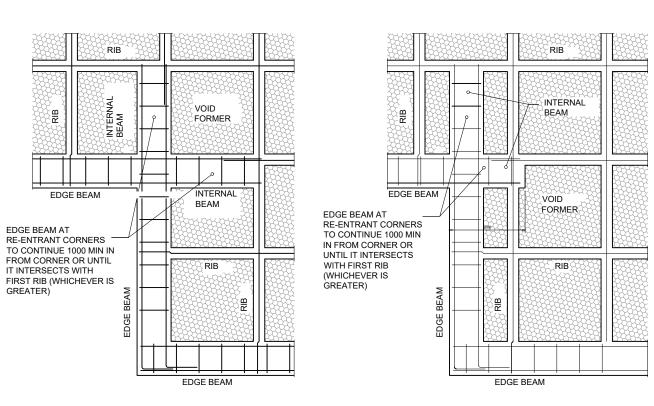
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PROPOSED ATTACHED DUAL OCCUPANCY
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939
GROUND FLOOR SLAB DETAILS (SHEET 2)

NOTE: DO NOT SCALE OFF DRAWINGS, REFER TO ARCHITECTURAL PLANS, VERIFY DIMENSIONS ON SITE





#### **ALIGN WITH RIBS**

#### **OFFSET WITH RIBS**

#### TYPICAL WAFFLE POD LAYOUT AT RE-ENTRANT CORNERS



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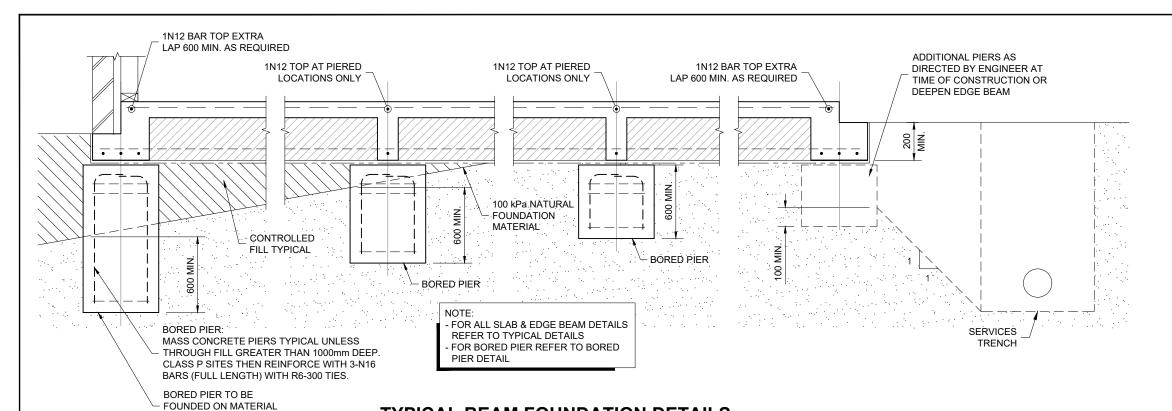
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NEMCODESIGN Pty. Ltd.	E: abraham@nemc
STRUCTURAL ENGINEERI & ARCHITECTURAL DESIG	

PROPOSED ATTACHED DUAL L 1, 402-410 IKSTOWN, 166 FLUSHCOMBE RD, BLA LOT 2, DP 38939 0422 606 228 ncodesign.com.a **GROUND FLOO** 

POSED ATTACHED DUAL OCCUPANCY	24244	S13	A
6 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	DESIGNED BY: A.N.	DATE: 03.09.2024	<
GROUND FLOOR SLAB DETAILS (SHEET 3)	DRAWN BY: A.N.	SCALE: AS SHOWN	

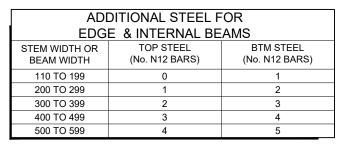
NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE

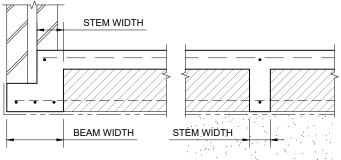


TYPICAL BEAM FOUNDATION DETAILS

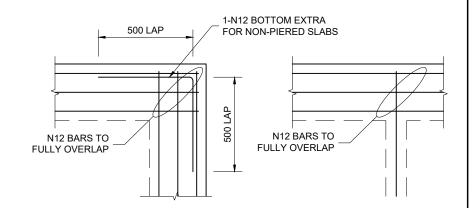
1N12 BAR TO TRENCH MESH & ADDITIONAL

BARS, TYPICAL

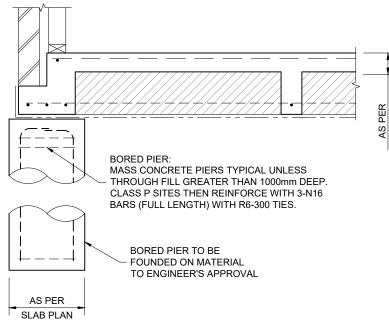




# **ADDITIONAL STEEL FOR**



# **EDGE & INTERNAL BEAMS**

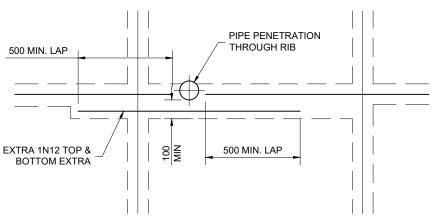


**BORED PIER DETAIL** 

**END LAP** 

TO ENGINEER'S APPROVAL

500 LAP



TYPICAL FOOTING CORNER DETAILS

N12 "L" BARS

TOP & BOTTOM

# WAFFLE POD DETAILS AT PENETRATION

## WAFFLE POD LAP DETAILS

#### **BORED PIER NOTE**

BORED PIERS SHALL BE USED IN ACCORDANCE WITH THE FOLLOWING:

- SET OUT AS PER THE ADJACENT PLAN.
- FOUNDED OFF VERY STIFF CLAY THAT IS UNIFORM & STABLE THROUGHOUT.
- CONC. PIERS TO BE FOUNDED TO A MINIMUM OF 250 kPa BEARING CAPACITY. GEOTECH TO VERIFY ON SITE.
- WHERE ROCK IS ENCOUNTERED, ALL PIER TO BE FOUNDED OFF ROCK THAT IS UNIFORM & STABLE WITH A MINIMUM OF **700 kPa** BEARING CAPACITY
- WHERE PIER LENGTH EXCEEDS 3000MM,
- REINFORCE PIERS WITH 5N16 VERT + R10-300TIES. WHERE FOOTINGS AND FOOTING BEAMS FOUND PARTLY ON ROCK AND PARTLY ON CLAY OR FILL, THE SECTION OF BUILDING NOT FOUNDED ON ROCK SHALL BE PIERED TO ROCK.



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REV	DATE	DESCRIPTION	BY	l

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N12 "L" BARS

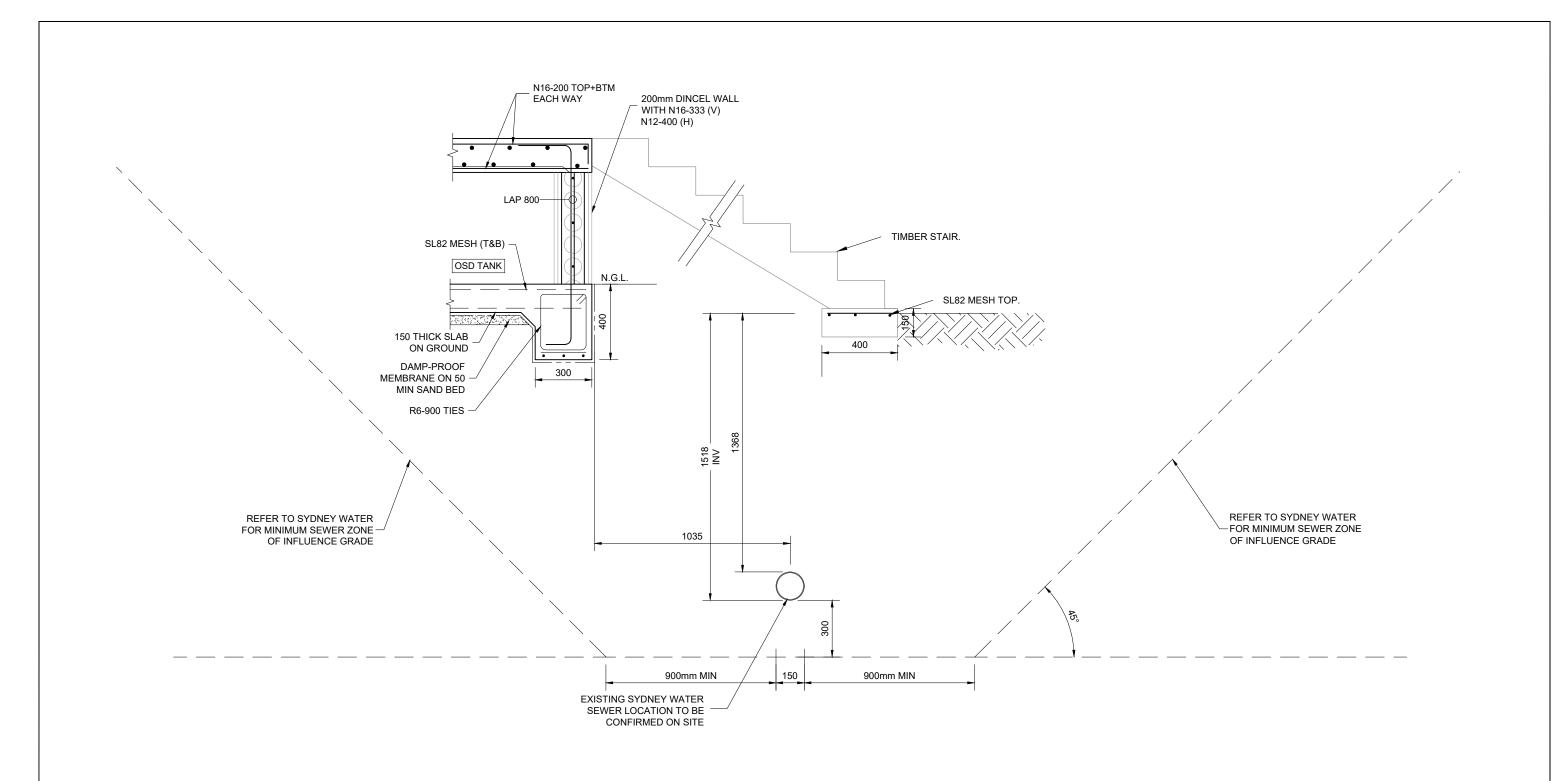
TOP & BOTTOM

PROPOSED ATTACHED DUAL OCCUPANCY 606 228

1110	OSED ATTACHED BOAL OCCOT ANOT
16	6 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939
	GROUND FLOOR SLAB DETAILS (SHEET 4)

JOB NUMBER:	DWG NUMBER:	ORIGINAL SIZE:
24244	S14	A3
DESIGNED BY:	DATE:	$\overline{A}$
A.N.	03.09.2024	
DRAWN BY: A.N.	SCALE: AS SHOWN	
A.IV.	AO ONOWN	

# NOTE: DO NOT SCALE OFF DRAWINGS, REFER TO





- MESH END AND SIDE LAPS 200mm NOMINAL.
   ADJACENT MESH RUNS TO BE SIDE LAP NESTED AND PLACED HALF SHEET LAP STRETCHER PATTERN.
- DESIGN LOADING AT FOOTING LEVEL NOT **EXCEEDING 50KPA**

	REVISION
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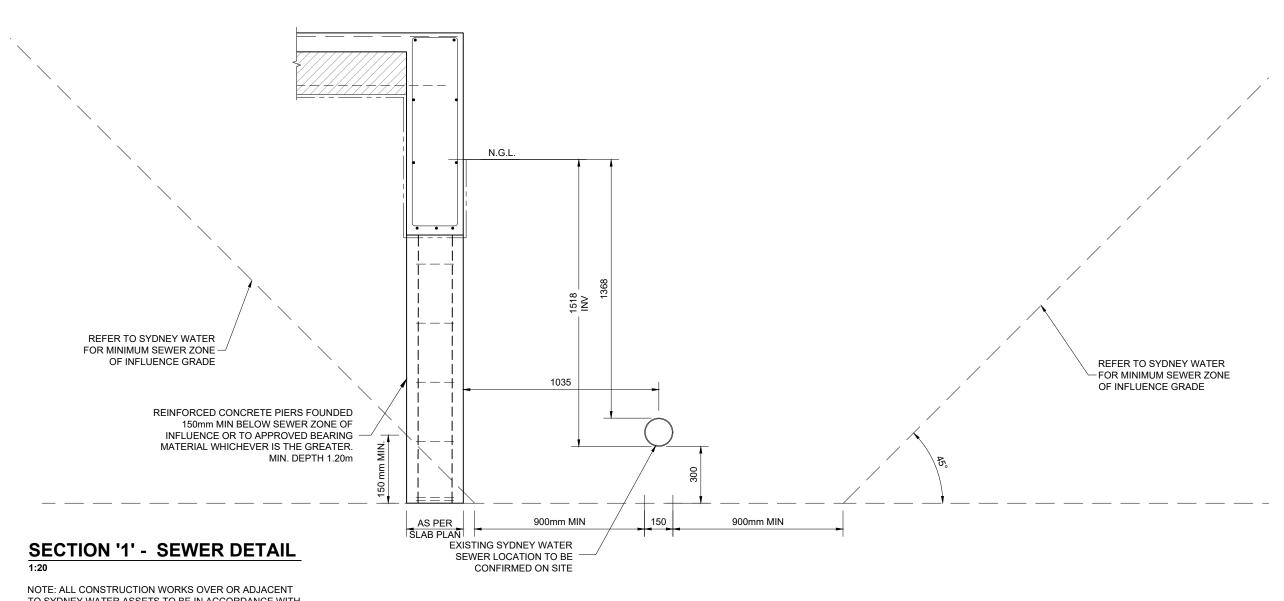
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	•	
emcodesign.com.au		
	STAIR DETAIL AND SEWER SECTION	

PROPOSED ATTACHED DUAL OCCUPANCY S15 A3 24244 166 FLUSHCOMBE RD, BLACKTOWN DESIGNED BY: LOT 2, DP 38939 03.09.2024 SCALE: AS SHOWN



NOTE: ALL CONSTRUCTION WORKS OVER OR ADJACENT TO SYDNEY WATER ASSETS TO BE IN ACCORDANCE WITH SYDNEY WATER REQUIREMENTS.

NOTE: DO NOT SCALE OFF DRAWINGS. REFER TO ARCHITECTURAL PLANS. VERIFY DIMENSIONS ON SITE

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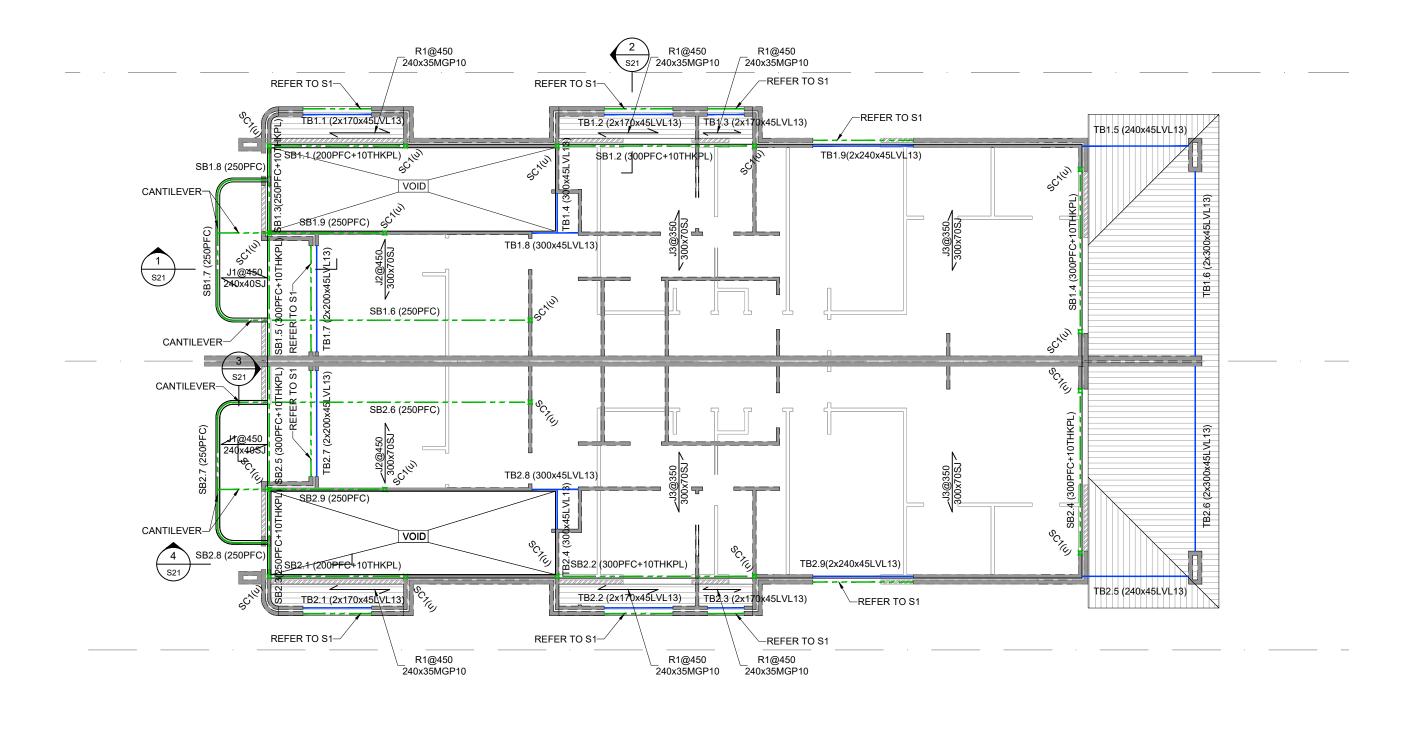
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& ARCHITECTURAL DESIGN

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CHAPEL RD, BANKSTOWN,
NSW 2200
P: 9709 5556 M: 0422 606 228
E: abraham@nemcodesign.com.au

OPOSED ATTACHED DUAL OCCUPANCY	JOB NUMBER: 24244	DWG NUMBER: \$16	ORIGINAL SIZE:
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	DESIGNED BY: A.N.	DATE: 03.09.2024	
SEWER SECTION	DRAWN BY: A.N.	SCALE: AS SHOWN	



#### FIRST FLOOR FRAMING PLAN

#### l:100

#### NOTES:

- 1. SC1 STEEL COLUMN 89 x 89 x 5 SHS.
- ALL BEAMS TO BE SUPPORTED BY TRIPLE STUDS IF NO HARDWOOD POST IS DENOTED ON THE PLAN.
- 3. ALL LOAD BEARING WALLS CARRYING ROOF TO BE SUPPORTED BY DOUBLE PARALLEL JOISTS.
- ALL OPENINGS ABOVE 900mm ARE TO BE SUPPORTED BY DOUBLE STUD ON EITHER SIDE.

	REVISION	
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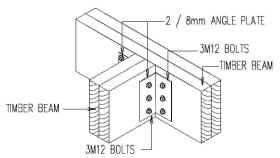
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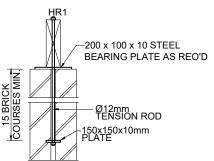
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PROPOSED ATTACHED DUAL OCCUPANCY	1
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	
FIRST FLOOR FRAMING PLAN	

Υ	JOB NUMBER: 24244	DWG NUMBER: S20	ORIGINAL A
	DESIGNED BY: A.N.	DATE: 03.09.2024	$\bigcap$
	DRAWN BY: A.N.	SCALE: AS SHOWN	





# **HOLD DOWN ROD**





#### 'B' = 150 2/ HILTI HVU M12 CHEMICAL 'C' = 200 CAPSULE WITH HAS-E ROD 'D' = 300 (ZINC PLATED), STANDARD 10mm STIFFENER 110mm EMBEDMENT PLATE 20 MORTAR BED STEEL BEAM **TYPICAL TIMBER BEAM**

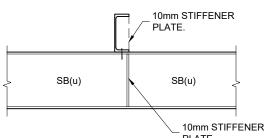
#### S1 - NON LOAD BEARING BRICK LINTEL SCHEDULE LINTEL SIZE SPAN MIN. END BEARING 90 x 90 x 6.0 EA **UPTO 2000** 150 OVER 2001 - 2400 90 x 90 x 8.0 EA 150 OVER 2401 - 2700 100 x 100 x 6.0 EA 150 OVER 2701 - 2900 150 100 x 100 x 8.0 EA OVER 2901 - 3300 150 x 90 x 8.0 UA 150 OVER 3301 - 3600 150 x 100 x 10.0 UA 150

NOTE: ALL EXTERNAL STEEL TO BE HOT DIPPED GALVANISED MAXIMUM HEIGHT OF BRICKWORK OVER LINTEL = 3000

TIMBER FRAMING MEMBER SCHEDULE			
MEMBER	SIZE		
BOTTOM PLATE	1/50 x 100 F7		
WALL STUDS	100 x 38 F7 AT 600 MAX CTS *		
TOP PLATE	2/50 x 100 F7		
RAFTERS	REFER TO PLAN		
BATTENS	35 x 70 F7		

\* PROVIDE TRIPLE NAIL LAMINATED STUDS AT EACH SIDE OF WINDOWS AND DOORS

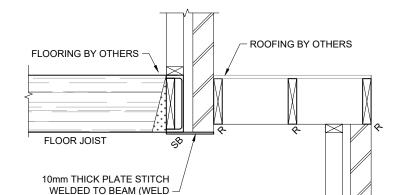
## BALUSTRADE BY **BALCONY FLOORING** BY OTHERS FLOORING BY OTHERS 47 FLOOR JOIST 10mm THICK PLATE STITCH WELDED TO BEAM (WELD 150, MISS 150)



		_Ц		
>	SB(u)		SB(u)	
			10mm : PLATE	STIFFENE

SECTION	3
1:20	S20

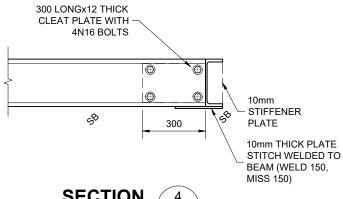
TIMBER FRAMING CONNECTION SCHEDULE			
MEMBER	CONNECTION	NAILING	
BATTENS TO RAFTERS	-	2/75 x 3.05 DEFORMED SHANK NAILS	
RAFTERS TO TOP PLATE OR BEAM	1 FRAMING ANCHOR	3/Ø2.8 NAILS TO EACH MEMBER	
LINTELS TO STUDS	1 LOOPED 30 x 0.8 G.I. STRAP 3/Ø2.8 NAILS TO EACH MEME		
TOP PLATE TO STUDS	1 LOOPED 30 x 0.8 G.I. STRAP TO EVERY THIRD STUD	3/Ø2.8 NAILS TO EACH MEMBER	
STUDS TO BOTTOM PLATE	1 LOOPED 30 x 0.8 G.I. STRAP TO EACH STUD STRAPPED TO TOP PLATE	3/Ø2.8 NAILS TO EACH MEMBER	
BOTTOM PLATE TO SLAB	M12 DYNABOLT WITHIN 100mm OF EACH STRAPPED STUD	-	
BOTTOM PLATE TO JOIST	1 LOOPED 30 x 0.8 G.I. STRAP TO AT EACH STRAPPED STUD	3/Ø2.8 NAILS TO EACH MEMBER	

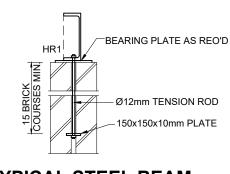


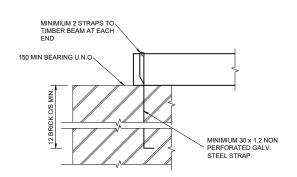
150, MISS 150)

**SECTION** 

S20







# **SECTION**

SECTION	4
1:20	S20

**TYPICAL STEEL BEAM HOLD DOWN ROD** 

		BEAM	TIE [	OOWN	DETAILS	IN	MASONRY
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E: abrahan

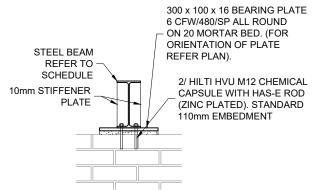
STRUCTURAL ENGINEERING

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CHAPEL RD, BANKSTOWN,
NSW 2200
P: 9709 5556 M: 0422 606 228
E: abraham@nemcodesign.com.au

PROPOSED ATTACHED DUAL OCCUPANCY		
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	DESIGN A N	
FIRST FLOOR FRAMING DETAILS	DRAWN A.N.	

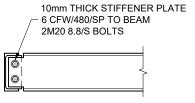
JOB NUMBER: 24244	DWG NUMBER: S21	ORIGINAL SIZE:
DESIGNED BY: A.N.	DATE: 03.09.2024	
DRAWN BY: A.N.	SCALE: AS SHOWN	

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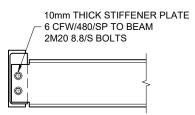
# **TYPICAL BEARING PLATE DETAIL**





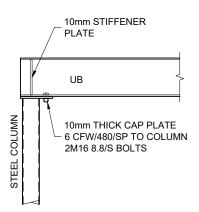
### STEEL BEAM CONNECTION - 200 PFC TO 200 PFC

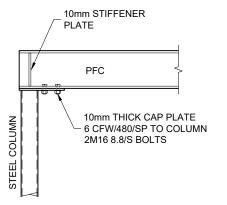
(CONNECTION CAPACITY 55 kN ULTIMATE) ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND

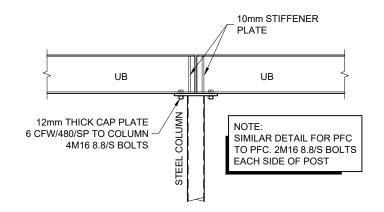


## LARGER BEAM SUPPORTING SMALLER BEAM

SMALLER BEAM MAX. SIZE = 250 PFC (CONNECTION CAPACITY 100 kN ULTIMATE) ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND

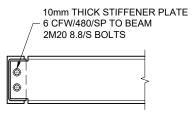






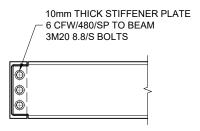
### **TYPICAL COLUMN CAP DETAILS**

BEAMS TO BE LOCATED CENTRALLY OVER COLUMN TYPICAL ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND



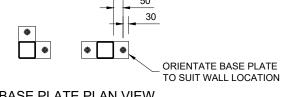
### STEEL BEAM CONNECTION - 250 PFC TO 250 PFC

(CONNECTION CAPACITY 100 kN ULTIMATE) ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND

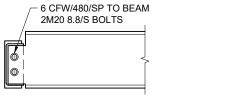


### STEEL BEAM CONNECTION - 300 PFC TO 300 PFC

(CONNECTION CAPACITY 100 kN ULTIMATE) ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND



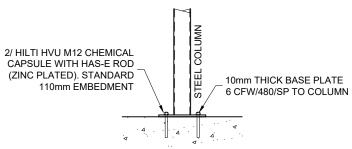
BASE PLATE PLAN VIEW



10mm THICK STIFFENER PLATE

### **SMALLER BEAM SUPPORTING LARGER BEAM**

LARGER BEAM MAX. SIZE = 300 PFC (CONNECTION CAPACITY 100 kN ULTIMATE) ALTERNATIVELY SITE WELD USING 6 CFW/480/SP ALL ROUND



# TYPICAL COLUMN BASE DETAIL



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1, 402-410 (STOWN, 422 606 228

TYPICAL STEEL DETAILS	
166 FLUSHCOMBE RD, BLACKTOWN LOT 2, DP 38939	
PROPOSED ATTACHED DUAL OCCUPANCY	

JOB NUMBER:	DWG NUMBER:	ORIGINAL SIZE:
24244	S30	A3
DESIGNED BY:	DATE:	$\overline{A}$
A.N.	03.09.2024	
DRAWN BY: A.N.	SCALE: AS SHOWN	

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